

Preliminary Report on the Effect of Pre-Boring on the Mobilized Friction Capacity of Pile Foundation Hydraulically Jacked into Expansive Soil

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Abstract: Pre-drilling was performed to reduce lateral earth pressure generated by pile foundation hydraulically jacked into expansive soil. Nineteen prestressed-precast spun concrete piles with diameter of 800 mm were penetrated into expansive soil up the depth of 40 m. Pre-drilling with diameter of 700 mm was carried out up to the depth of 12.5 m. Penetration loads required to install the piles, which were displayed on the built-in pressure panel, were recorded every 1 m interval. The load that merely consists of shaft friction is then used to find out the mobilized skin friction between pile shaft and its surrounding soils. The calculated mobilized skin friction is correlated to the value of Standard Penetration Test (N_{SPR}) and compared to that computed by Decourt formula. The result shows that skin friction calculated using Decourt formula relatively conservative compared to those determined from the records of field penetration. The upper bound of mobilized skin friction can be defined by modifying Decourt formula.

Keywords: Expansive soils, jack-in piles, shaft friction.

Introduction

Installation of pile foundations using displacement methods such as driven pile and jacked-in pile might affect its neighboring structures due to lateral earth pressure generated by movement of surrounding soils. The effect of piling activity to the surrounding soil diminishes beyond 7 times of pile diameter from the pile [1]. In other words, the soil and structures in the range of 7 times diameter of the pile will be affected by installation of displacement pile foundations.

Pre-boring is a common method to reduce the undesirable effect due to lateral earth movement (pressure) generated by installation of driven piles or jacked-in pile foundation. The diameter of pre-drilling has to be smaller than that of the pile foundation so that the skin friction between soil and pile shaft remains unchanged. Broms [2] stated that for friction piles in clays, the outside diameter of the pre-drilling must be four centimeter less than that of the pile so that the friction resistance between soil and pile shaft unaffected by pre-drilling. Queensland Department of Transport and Main Roads [3] stated that in order to maintain the friction capacity of pile foundations, the difference between pile and pre-drilling diameter must be larger than five centimeter.

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Case Study

Pre-drilling method is being carried out to install 800 mm diameter pre-stressed precast concrete spun pile into expansive soil layer in West Surabaya, Indonesia. The expansive potential of soil layers is presented in Table 1. A Continuous Flight Auger (CFA) with diameter of 700 mm was used to pre-drill soil column up to the depth of 12.5 m. Out of about 550 piles scheduled to be installed, three piles with 40 m length were hydraulically jacked into the 40 m depth, eight 26 m piles were installed up to the depth of 40 m using follower with the same diameter, and eight 36 m piles were jacked into 36 m deep.

The hydraulic jack-in machine (type JYV 700) that is used to install the piles was manufactured by CHTC Jove Heavy Industry Co, LTD – China. The machine exhibits maximum penetration capacity of 700 ton, maximum rate of penetration (pile pressing speed) of 1.48 m per minute, and penetration stroke of 1.80 m [4]. The mobilized pressure to penetrate the piles was monitored and recorded each and every 1 m through built-in pressure gauge indicator that is displayed in the operational compartment of the machine.

Table 1. Heaving Potential

Depth, m	Heave, %	Bor
11.5 m – 12.0 m	1.3	B1
15.0 m – 16.0 m	3.3	B1
21.5 m – 22.0 m	3.3	B1
13.0 m – 13.5 m	2.3	B2
17.0 m – 17.5 m	1.8	B2
17.5 m – 18.0 m	3.5	B3
23.5 m – 24.0 m	3.6	B3
13.0 m – 13.7 m	1.2	B4
17.5 m – 18.0 m	2.9	B4
21.5 m – 22.0 m	3.8	B5

Soil Data

Soil investigation [5] that consists of five borings was performed up to the depth of 60 m. It was reported that the undrained shear strength of soil increases with depth (Figure 1), with average undrained shear strength of 4 ton/m². The value of Standard Penetration Test (N_{SPT}) that was conducted at every 2 m interval is presented in Figure 2. In general, the N_{SPT} increases linearly with depth. Figure 2 also shows that soil layers in the location of the project are relatively homogenous. For the purpose of clarity, the Uncorrected Standard Penetration Test (N_{SPT}) up the depth of 14 m is presented in Figure 3. The mean Standard Penetration Test of 14 m thick of upper layer soil can be correlated linearly with depth, as shown in Figure 3.

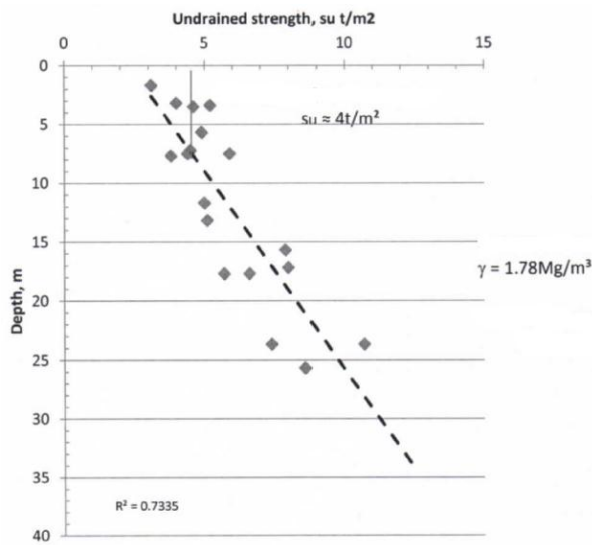


Figure 1. Undrained Shear Strength (Su) of Soil Layers

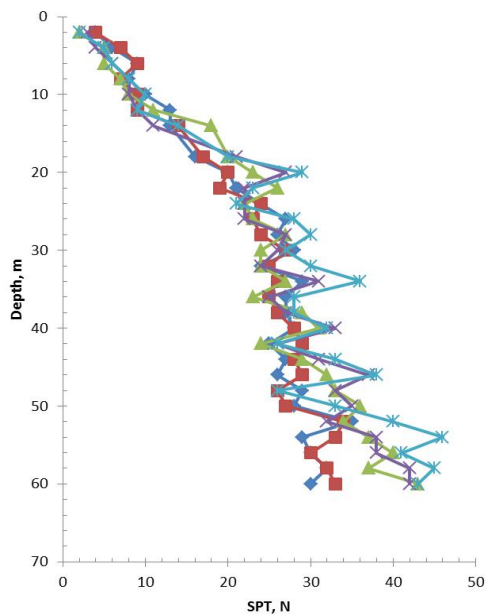


Figure 2. Uncorrected Standard Penetration Test (N_{SPT})

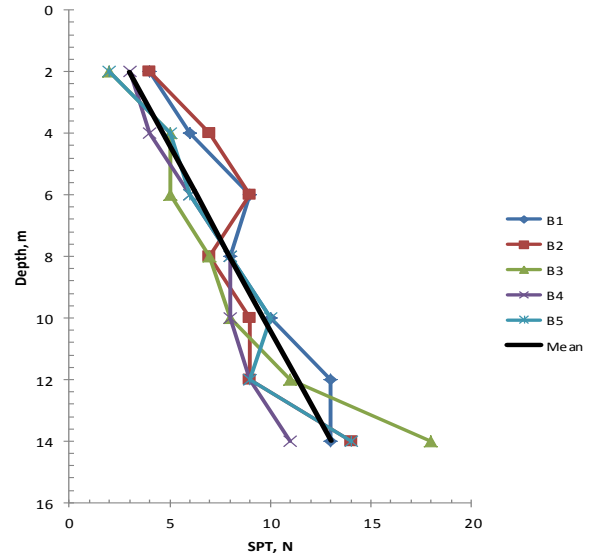


Figure 3. Average Uncorrected Standard Penetration Test (N_{SPT})

Penetration Load Data

Figure 4 shows the loads applied to install the piles. Maximum load required to penetrate 19 piles ranging from 540 ton to 640 ton. While Figure 5 shows the penetration load required to install the first 12 m of the piles, where the bearing capacity of piles comprises of merely on shaft friction.

Mobilized Shaft Friction (fs)

Mobilized unit skin friction of piles (fs) is determined by averaging load of penetration at every 2 m interval divided by surface area of the pile at the same interval. The first 3 m deep of the penetration data are omitted since the soil at this layer consisted of non-expansive fill material.

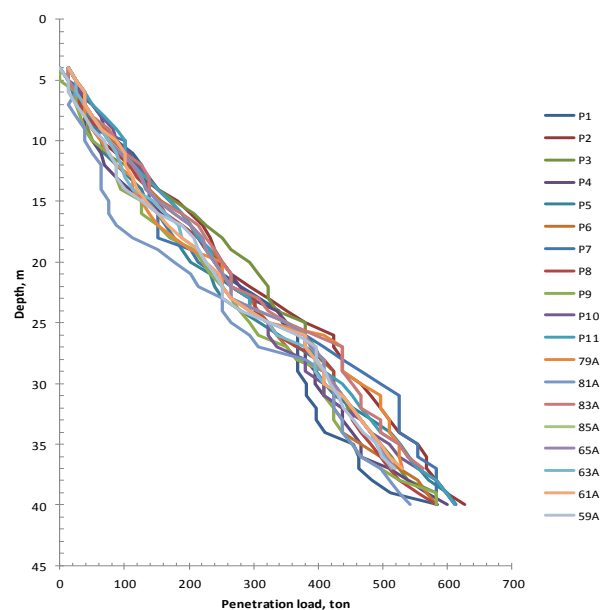


Figure 4. Load of Penetration with Depth

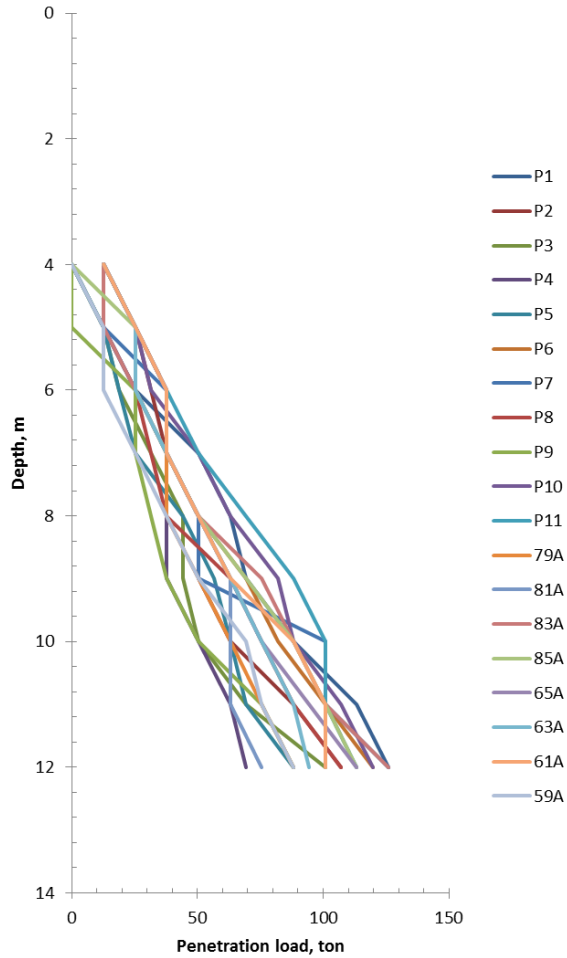


Figure 5. Load of Penetration Required to Install the First 12.5 m of the Piles

Figure 6 shows the mobilized unit shaft friction of the piles up 12 m deep. It shows that, in general, the mobilized unit shaft friction of the piles increases with depth. However, the variation of calculated mobilized unit shaft friction is quite large. Comparing the calculated unit shaft frictions to those proposed by Decourt [6] and Schmertmann [7], which are expressed in Equation 1 and Equation 2 respectively, the unit shaft friction stated by both Decourt and Schmertmann are within the calculated mobilized shaft friction, as shown in Figure 6.

The mean value of Standard Penetration Test of five borings up to 14 m deep, which is expressed in Equation 3, is used to calculate the unit shaft friction proposed by Decourt and Schmertmann formulas;

$$f_s = \frac{N_{SPT}}{3} + 1 \text{ [ton/m}^2\text{]} \quad (1)$$

$$f_s = 0.5 N_{SPT} \text{ [ton/m}^2\text{]} \quad (2)$$

$$N_{SPT} = 1.1932(z) - 1.58, \quad \text{where } z \text{ represents the depth} \quad (3)$$

Bromham and Styles [8] mentioned that Meyerhof formula for sands [9], which is presented in Equation

4, could also be used to predict shaft resistance for stiff clays;

$$f_s = \frac{N_{SPT}}{5} \text{ [ton/m}^2\text{]} \quad (4)$$

However, the skin friction determined using Meyerhof formula (Equation 4) is more conservative compared to that proposed by Schmertmann (Equation 2)

Figure 6 also shows that Decourt formula considered as lower limit of mobilized shaft friction. Trial and error on Decourt formula was exercised to determine the upper bound of calculated unit skin friction. The upper bound of the calculated mobilized unit skin friction could be expressed as;

$$f_s = \frac{N_{SPT}}{3} + 4 \text{ ton/m}^2 \quad (5)$$

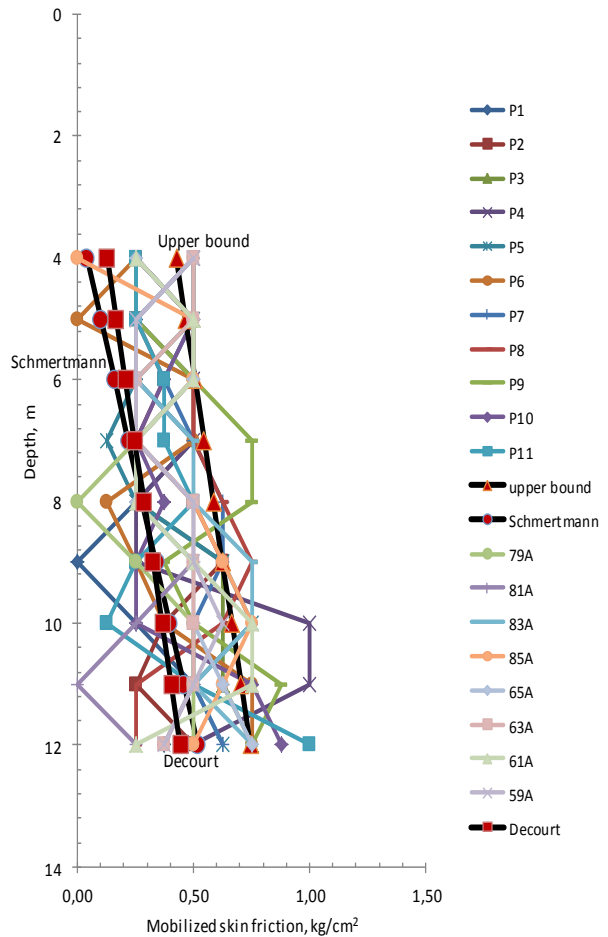


Figure 6. Mobilized Unit Skin Friction

Conclusion

Based on limited number of data, the preliminary research on shaft friction shows that the mobilized skin friction of concrete pile foundation hydraulically jacked into expansive soil can be determined conservatively using Decourt formula. The upper bound of mobilized skin friction can be calculated using modified Decourt formula, as shown in (Equation 5).

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